Conité français des barrages et réservoirs International Symposium Qualification of dynamic analyses of dams and their equipments and of probabilistic assessment seismic hazard in Europe 31th August – 2nd September 2016 – Saint-Malo

> Luc Boutonnier Dino Mahmutovic



Session : Qualification of seismic analysis of embankment dams Dynamic analysis of Aratozawa dam including the effect of occluded air and pore fluid compressibility



SUMMARY

1.Introduction

2. Presentation of Aratozawa Dam

3. Dynamic analysis of Aratozawa dam

4.Conclusion



Introduction

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Earthquakes and Japan

Illustration of tectonic plate movement in Japan





Japan \rightarrow an area of high seismicity with a long history of earthquake



Many dams in Japan

Localisation of main dams in Japan



Map of all types of dams

HELDNGJIANG

Map of earthfill dams

Japan \rightarrow many acceleration history measurements available during earthquakes



JCOLD / CFBR collaboration

Share data and experiences in design



Improve the calculation procedures using data from real seismic events with high energy earthquake



Aratozawa Dam

Localisation of Aratozawa dam





http://maps.ontarget.cc/dams/en.html



Localisation of Aratozawa dam



http://maps.ontarget.cc/dams/en.html



Geometry of Aratozawa dam (Ohmachi and Tahara, 2011)





Main earthquake (Ohmachi and Tahara, 2011)

No	Date (time)	Magnitude M	Peak acceleration at gallery (m/s ²)
1	1996.8.11(3:12)	5.9	0.28
2	1996.8.11(8:10)	5.7	0.33
· · 3	1996.8.11(15:01)	4.8	Sul kity for the 0.30 (Anterwork)
4	2008.6.14(8:43)	7.2	10.24
5	2008.6.14(9:00)	4.7	0.99
5 6	2008.6.14(* 9:01)	4.0	4.82
16	2008.6.14(12:10)	4.7	0.79
19	2008.6.14(19:11)	4.1	2.29
- 36	2008.6.16	5.3	0.76
62	2008.6.19	3.2	0.36
118	2008.7.18	3.0	0.53
127	2008.7.24	•.•. 6,8 • • •	0.24
137	2008.7.29	3.9	0.90
149	2008.8.4	3.5	0.78
169	2008.9.25	4.1	1.19
176	2008.12.15	3.4	0.39
183	2009.7.1	3.2	0.02
189	2009.8.4	1.6	0.02



→ Estimation of seismic wave velocity Vs :

 $V_s = rac{distance\ between\ 2\ acceloremeters}{delay\ between\ 2\ acceloremeters}$



Main earthquake episods(Omashi 2011)

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At the end of the earthquake:





Main earthquake episods

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At the end of the earthquake:

u_w V_s

How to explain these phenomena?

Anisotropy?

Irreversible plasticity?



Construction





Main stages in the numerical model calculations

Store	Undroulie	Mechanical	Comment
Stage	Hyuraunc	calculation	



Construction



Numerical construction by step

Initial state of soil after compaction

$$\sigma'_{vini} = -u_{w,ini}$$

$$\sigma'_{h} = K0 * \sigma'_{v}$$
$$K0 = (1 - \sin(\phi')) * \left(\frac{\sigma'_{p}}{\sigma'_{vini}}\right)^{0.5}$$



Construction

Skeleton compressibility parameters in the core of the dam parameters



	λ	к	comment	
	0.14 0.025	PI=32.		
Eine soil			WL = 32/0.73+13 = 57 (estimated from	
Fille Soli		0.025	Casagrande, 1947)	
fraction of the		0.025	Correlations using WL and PI : Biarez and	
core		Favre (1975), Fleureau and al. (2002), Favre et al.		
			(2002)	
Real soil of the	Real soil of the $\lambda_{real soil} = \lambda_{fine soil fraction} * \mu_{cl}$		$\lambda_{\rm realsoil} = \lambda_{\rm finesoilfraction} * \mu_{\rm cl}$	
core	0.035	0.006	$\kappa_{real \ soil} = \kappa_{fine \ soil \ fraction} * \mu_{cl}$	
Assumption 1	Assumption 1 $\mu_{cl} = 25\%$		$\mu_{cl} = 25\%$	
Real soil of the $\lambda_{real soil} = \lambda_{fine soil fraction} * \mu_{cl}$		$\lambda_{realsoil} = \lambda_{finesoilfraction} * \mu_{cl}$		
core	0.14	0.025	25 $\kappa_{\text{real soil}} = \kappa_{\text{fine soil fraction}} * \mu_{\text{cl}}$	
Assumption 2			$\mu_{cl} = 100\%$	

→ p'c = 250 kPa (W_{OPN} + 1%)

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Construction

Skeleton compressibility parameters in the core of the dam parameters





Construction

Fluid compressibility



→ Prediction of pore pressure build-up in the core



Construction



$$c_f = \frac{1}{S_r} \cdot \frac{dS_r}{du_w} + c_w$$









Impoundement

Pore pressure set up in the numerical model





Construction



Elastic parameters

 $G_{\max} = \rho * V_s^2$

Values of V_s from Sawada and Takahashi (1975)

Depth z	Clay core V _s Clay core V		Saturated rockfill+	Unsaturated rockfill
	(lower bond)	(upper bond)	transition V _s	+ transition V_s
(m)	(m/s)	(m/s)	(m/s)	(m/s)
0-5m	$V_{s} = 210$		$V_{s} = 245$	
5-30m	$V_{-180} - 0.35$	V 140 - 0.34	$V_s = 250.z^{0.2}$	$V_{s} = 250.z^{0.2}$
>30m	v = 180.z	$v_s = 140.z$	$V_s = 200.z^{0.31}$	$V_s = 250.z^{0.2}$







Dynamic stage

Elastic parameters

$$G_{\max} = K_{\alpha} * \left(\frac{\sigma_3'}{p'_{ref}}\right)^{\alpha}$$
 Use of a power law with minor effective stress σ_3' in order to obtain a value of Gmax depending on the depht

Comparison between V_s from Sawada and Takahashi (1975) and V_s calculated from $G_{max}(\sigma'_3)$

















Measured spectral ratio

Calculated spectral ratio





Estimation of pore pressure excess





Estimation of shear wave velocity Vs



p'c = 150 kPa - μ_{cl} = 100%

*** Vs measured ooo Vs_G calculated with distortion - Vs_Gmax calculated without distortion



Estimation of Settlements at the end of earthquake (before consolidation)





Effect of the occluded air





Conclusion

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Conclusion

- Attenuation of acceleration at mid height and at the crest of the dam = hysteretic damping + irreversible plasticity during strong earthquake;
- u_w = the pore fluid compressibility (occluded air) + isotropic hardening in the fine soil fraction of the core during earthquake;
- >> G_{max} & V_s <==> ---- uw
- $V_s(t) = V_s(\gamma, p'(u_w))$
- Irreversible settlement = isotropic hardening during earthquake + differed settlement during dissipation of pore pressure

THANK YOU FOR YOUR ATTENTION











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